# **Groundwater Hydraulics**

Institute for Fluid Mechanics and Environmental Physics in Civil Engineering, Universität Hannover





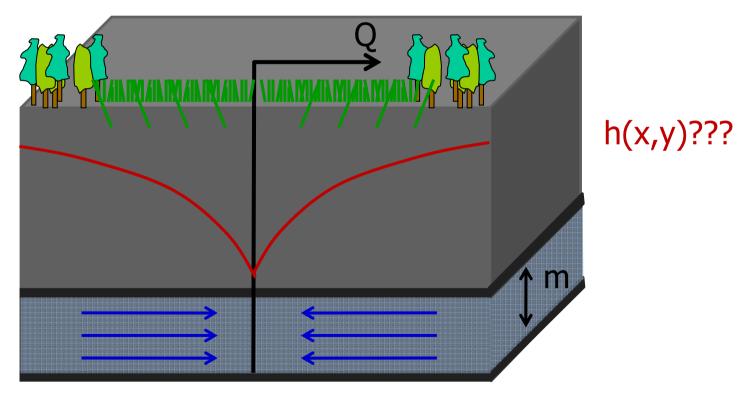
# **Steady state well flow**





## Steady state flow towards a well Confined aquifer

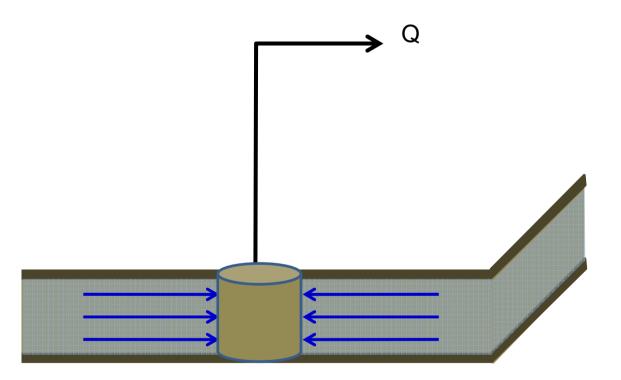
Vertical cut







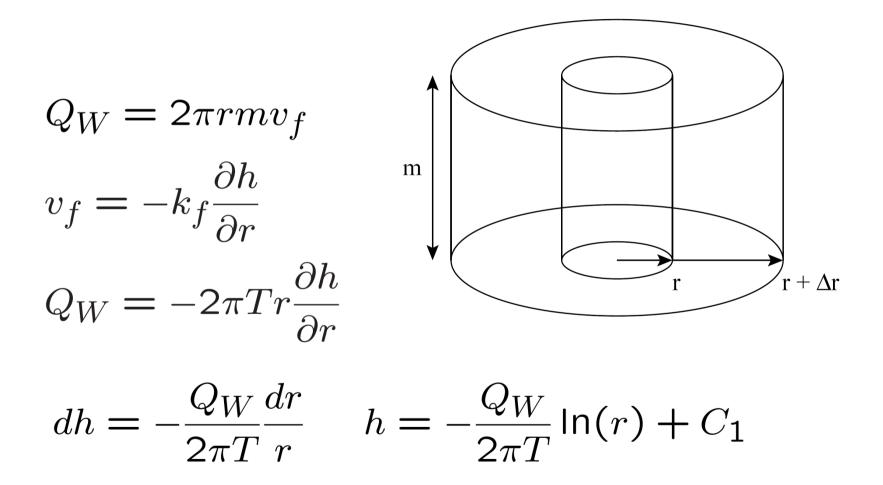
#### Flow through a cylinder:







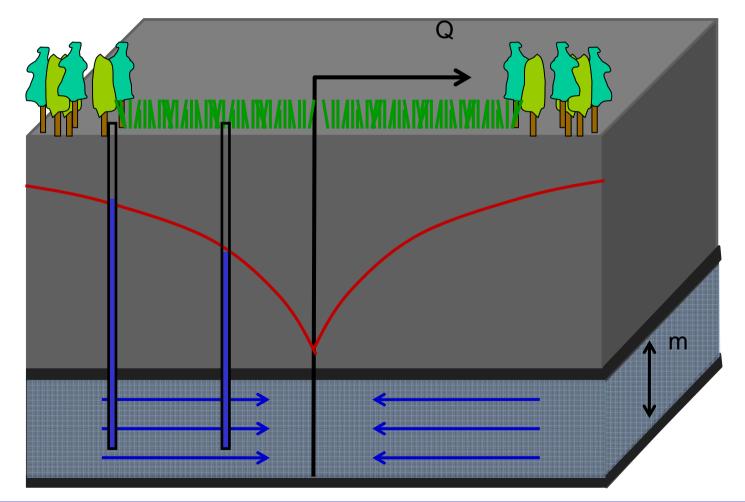
#### **Calculation of the head distribution:**



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#### **Dupuit-Thiem (1906)**







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Observation of the piezometric head at to observation wells at distance  $r_1 \mbox{ and } r_2$ 

$$\frac{-Q_w}{2\pi T}\ln(r_1) = h(r_1) + C_1$$
$$\frac{-Q_w}{2\pi T}\ln(r_2) = h(r_2) + C_1$$

Taking the difference gives

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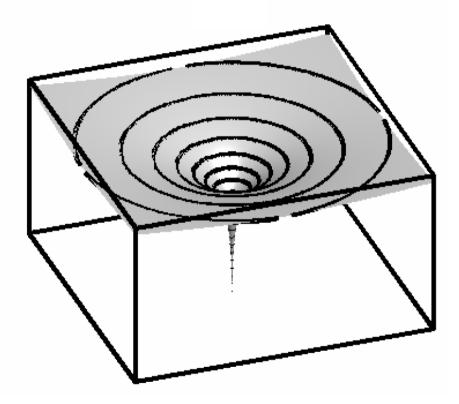
$$h(r_1) - h(r_2) = \frac{Q_w}{2\pi T} \ln\left(\frac{r_2}{r_1}\right)$$

Can be used to determine the transmissivity of an aquifer.



#### **Steady-State Drawdown**

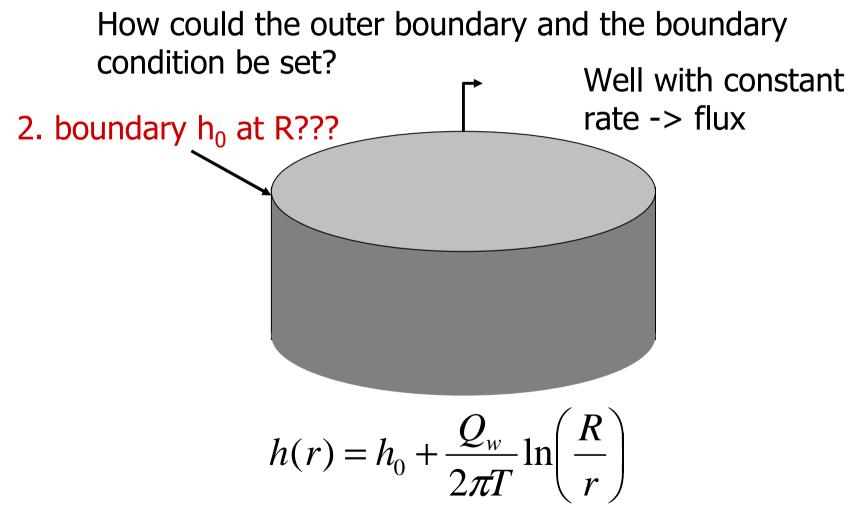
- Drawdown s = head difference without and with pumping
- "Cone of depression" for extraction well
- "Groundwater mounting" for injection well
- Logarithmic radial profile (without recharge)







#### **Radius of influence**





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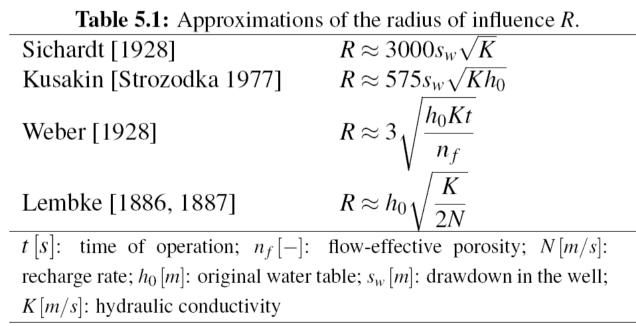
- Distance at which no drawdown occurs
- Strictly speaking, no radius of influence possible at steady state without recharge.

Environmetal Hydraulics, Groundwater, WS 2011/2012

• Empirical relationships:

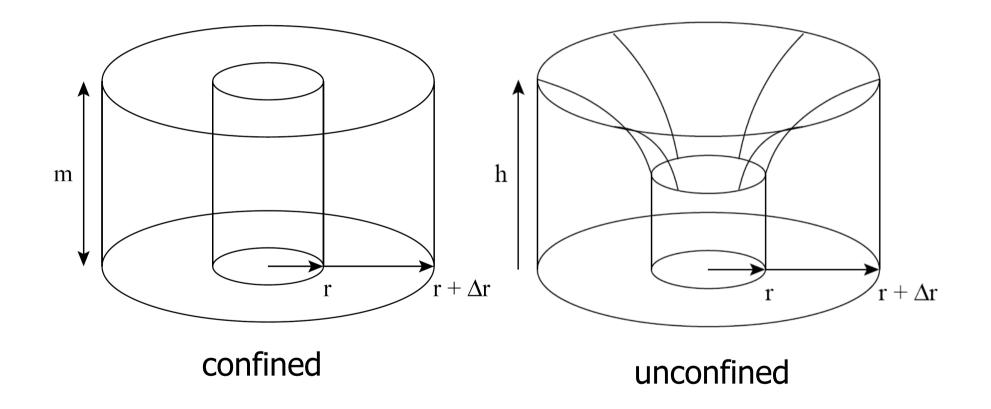
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#### Steady state flow towards a well Unconfined aquifer







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Without recharge, similar to confined:

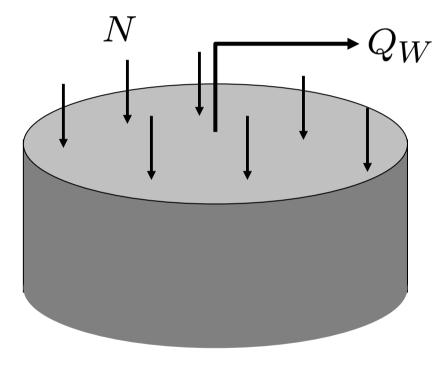
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$$Q(r) = Q_W = -\pi r k_f \frac{\partial h^2}{\partial r}$$
$$h^2 = -\frac{Q_W}{\pi k_f} \ln(r) + C_1$$
Difference: 
$$h_1^2 - h_2^2 = \frac{Q_W}{\pi k_f} \ln\left(\frac{r_2}{r_1}\right)$$



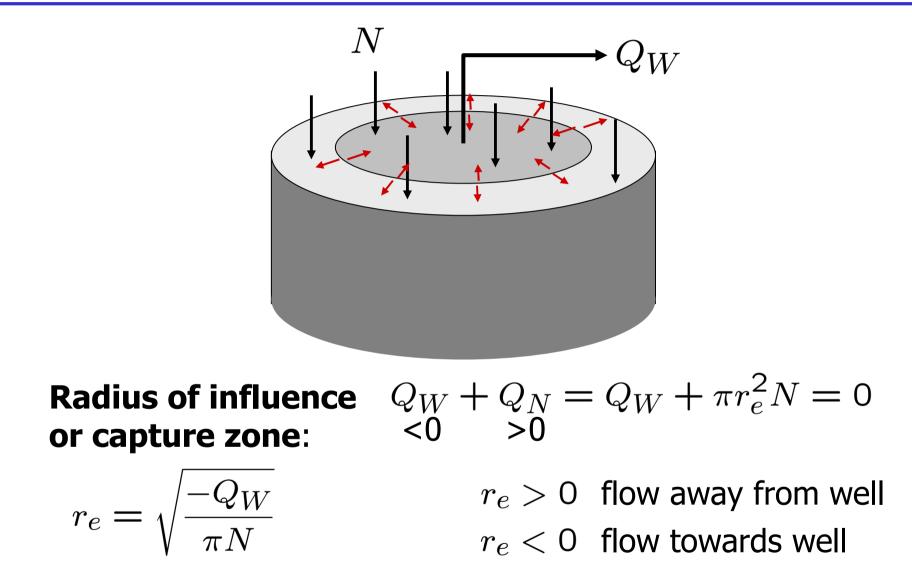
#### **Unconfined aquifer with recharge:**



 $Q(r) = Q_W + Q_N$ 











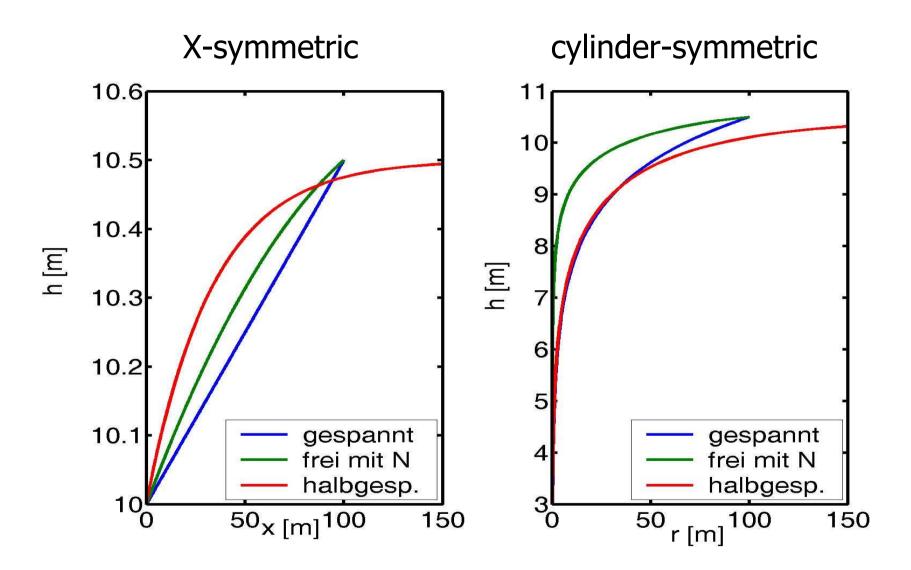
• Compare head squared at two distances

$$h_1^2 - h_2^2 = \frac{Q_w}{\pi K} \ln\left(\frac{r_2}{r_1}\right) + \frac{N}{2K} \left(r_2^2 - r_1^2\right)$$

- Logarithmic contribution from well
- Quadratic contribution from recharge
- Pumping well + recharge: capture zone
- No recharge and only one observation well: Radius of influence

$$h_0^2 - h_1^2 = \frac{Q_w}{\pi K} (\ln(r_1) - \ln(R))$$









#### Exercise # 11:

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Consider a sandy-gravely unconfined aquifer with a well extracting water with a rate of  $Q_w$  of 6.31 10<sup>-2</sup> m<sup>3</sup>/s. The hydraulic head without pumping is 8.2 m. In quasi steady state, the following hydraulic heads have been measured: r[m] = h[m] | r[m] = h[m]

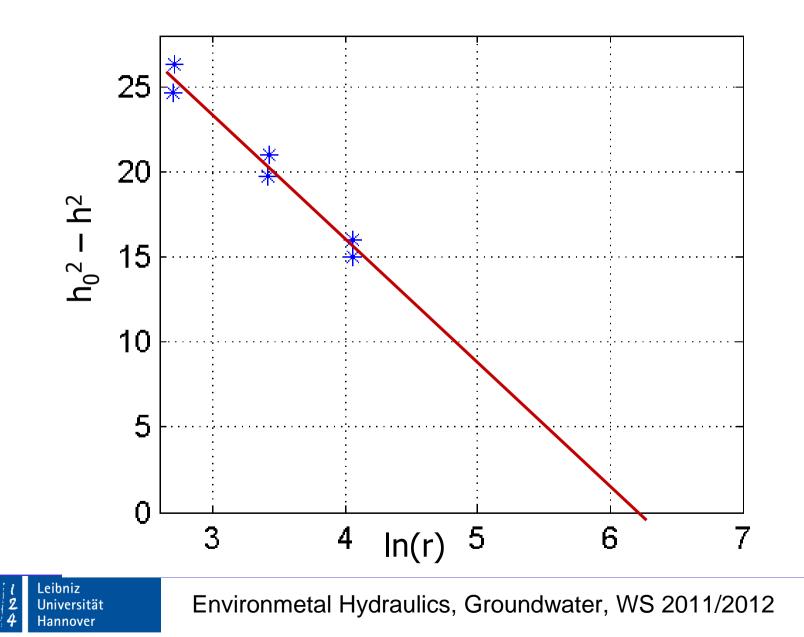
r [m]	h[m]	r [m]	h[m]
15	6.40	14.95	6.53
30.7	6.80	30.6	6.89
57.7	7.16	57.9	7.23

• Perform a linear regression of the difference between the squares of the undisturbed and measured head  $h_0^2$ - $h^2$  and the logarithm of the radius ln(r) and determine the hydraulic conductivity kf and the radius of influence of the aquifer. Careful: The rate  $Q_w$  has a negative sign if water is pumped from the well.

• Calculate for various radii of the well  $r_w$  (0.1 m, 0.25 m, 0.6 m, 1m) Sirchardt's maximum capacity and the maximum extraction rate possible in the given formation with the given well.

• Discuss constructive measures to obtain larger effective well diameters.







Read the slope: 
$$\frac{\Delta (h_0^2 - h^2)}{\Delta \ln(r)} \approx \frac{10m^2}{1.4} = 7.14m^2$$
  
 $sl = \frac{Q_W}{\pi k_f} = \frac{-6.31 \, 10^{-2} \, m^3/s}{\pi k_f}$   $k_f = 0.0026m/s$   
Read the x-axis for y=0:  $\ln(R) \approx 6.2$   $R = 490m$ 

